



<b>Project:</b> Glass Adaptor 1.5kN	<b>Contract:</b> 1419-1
<b>Subject:</b> Glass Balustrade	<b>Sheet No.</b> 1
<b>Date:</b> 23/07/2020	<b>By:</b> R.F.

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United Kingdom

### Glass Balustrade

1419-1 Glass Adaptor 1.5kN

Analysis By	Checked By
R.F.	T.S.

0	23/07/2020	T.S.	Issued
Revision	Date	Issued By	Comment



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### Introduction/Actions/Result Summary:

#### Introduction:

TSA was instructed by On Level to carry out calculations to be made with glass adaptor  $\phi$ 50mm thickness.



#### Actions:

Balustrade load = 1.5kN/m

(Table NA.5 IS1991-1-1:2002)

Point load = 1.5kN

(Table NA.4.2 IS 1991-1-1:2002)

Infill load = 1.5kN

(Table NA.5 IS1991-1-1:2002)

#### Assumption:

Concrete Grade = C30/37

#### Result Summary:

Study	Size of the Glass (l x h) (m)	Glass (mm)	Interlayer	Working Line Load for System (kN/m)	Glass Deflection (mm)
Case Study 01	1.1 x 1.45	21.52	Sentry	1.5	15.19
Case Study 02	1.5 x 1.45	21.52	Sentry	1.5	14.42

#### **NOTE:**

- All deflection < 25mm and therefore acceptable
- Glass thickness chosen determined by the material stress when subjected to 1.5kN/m Balustrade Load, 1.5kN/m<sup>2</sup> Infill Load and 1.5kN Point Load

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## Glass Strength

Balustrade Loading:

< 5mins duration =>  $k_{mod} = 0.77$

$$f_{gd} = (k_{mod})(k_{sp})(f_{gk}) / \gamma_{ma} + k_v(f_{bk} - f_{gk}) / \gamma_{mv}$$

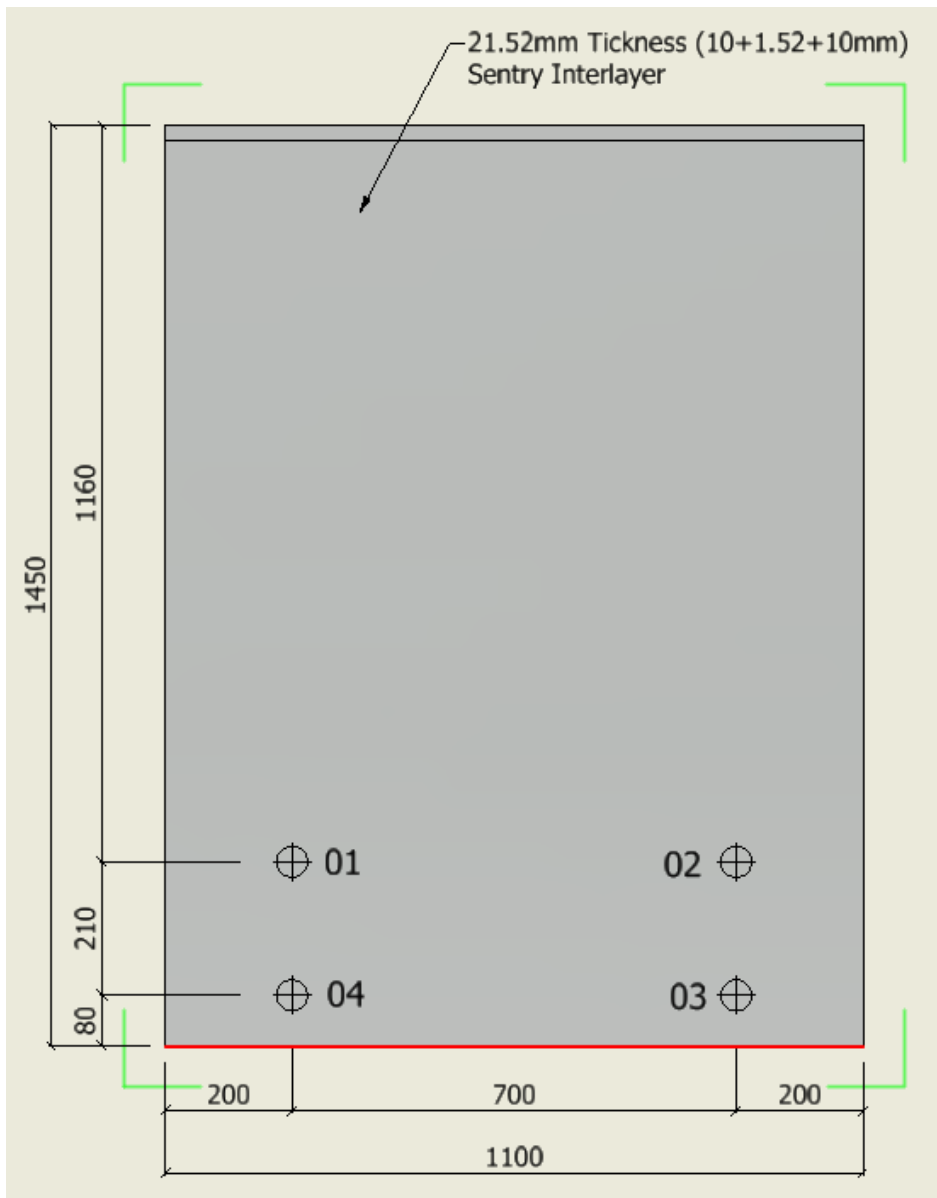
$$f_{gd} = (0.77)(1.0)(45) / 1.6 + 1.0(120 - 45) / 1.2$$

$$f_{gd} = \underline{84.2 \text{ N/mm}^2}$$

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### Case Study 01:

Sketch - 21.52mm thickness – 1.5kN/m - 1.1x1.45m – Sentry Interlayer:



### **NOTE:**

- Deflection on the glass 15.19mm = **OK in deflection**

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Glass Analysis – 21.52mm thickness – 1.5kN/m - 1.1x1.45m – Sentry Interlayer:

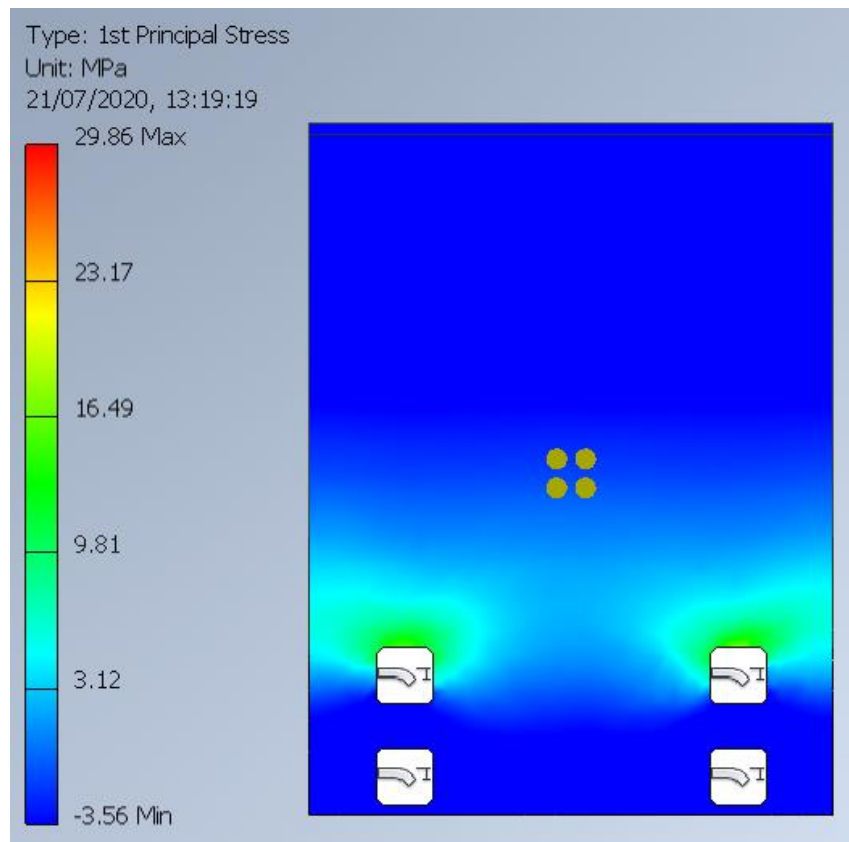
**Glass Analysis - Bending Stress of Glass Panel due to 1.5kN/m<sup>2</sup> Infill Loading:**

- Analysis Software was used to determine maximum bending stress of the glass due to 1.5N/m<sup>2</sup> Infill Loading
- 10/10/1.52mm T/L/T Glass analysed, horizontally toughened Laminated
- Interlayer Properties used for analysis, E= 606 MPa, G = 203.36MPa Sentry Glass SG5000
- Bending Stress analysed based on glass panel of 1.1m x 1.45m

**Result:**

Max. Bending Stress =  $29.86\text{N/mm}^2 \times 1.5 = 44.79\text{N/mm}^2 < 84.2\text{N/mm}^2$

**OK in Bending**



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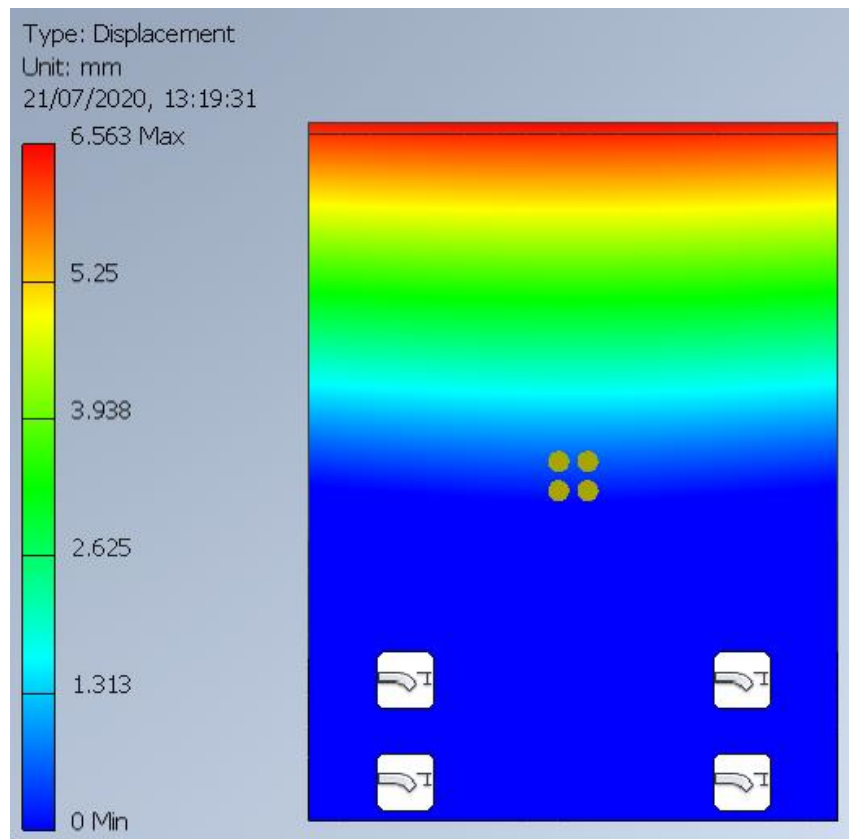
### Glass Analysis - Deflection of Glass Panel due to 1.5kN/m<sup>2</sup> Infill Loading:

- Analysis Software was used to determine maximum bending stress of the glass due to 1.5N/m<sup>2</sup> Infill Loading
- 10/10/1.52mm T/L/T Glass analysed, horizontally toughened Laminated
- Interlayer Properties used for analysis, E= 606 MPa, G = 203.36MPa Sentry Glass SG5000
- Deflection Stress analysed based on glass panel of 1.1m x 1.45m

### Result:

Max. Deflection = 6.563mm < 25mm {BS6180:2011 cl. 6.4.1}

**OK in Deflection (Glass Only)**



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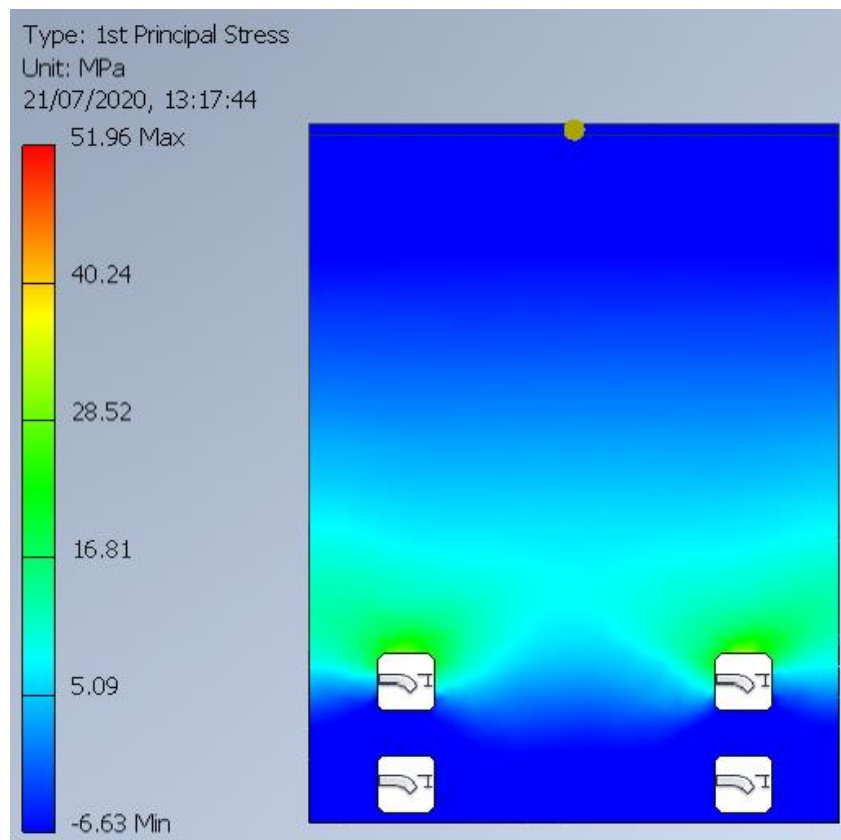
### Glass Analysis - Bending Stress of Glass Panel due to 1.5kN/m Balustrade Loading:

- Analysis Software was used to determine maximum bending stress of the glass due to 1.5kN/m Balustrade Loading
- Actual Balustrade Load applied to the glass is 1.65 kN/m (1.5x1.1m)
- 10/10/1.52mm T/L/T Glass analysed, horizontally toughened Laminated
- Interlayer Properties used for analysis, E= 606 MPa, G = 203.36MPa Sentry Glass SG5000
- Bending Stress analysed based on glass panel of 1.1m x 1.45m

### Result:

Max. Bending Stress =  $51.96\text{N/mm}^2 \times 1.5 = 77.94\text{N/mm}^2 < 84.2\text{N/mm}^2$

**OK in Bending**





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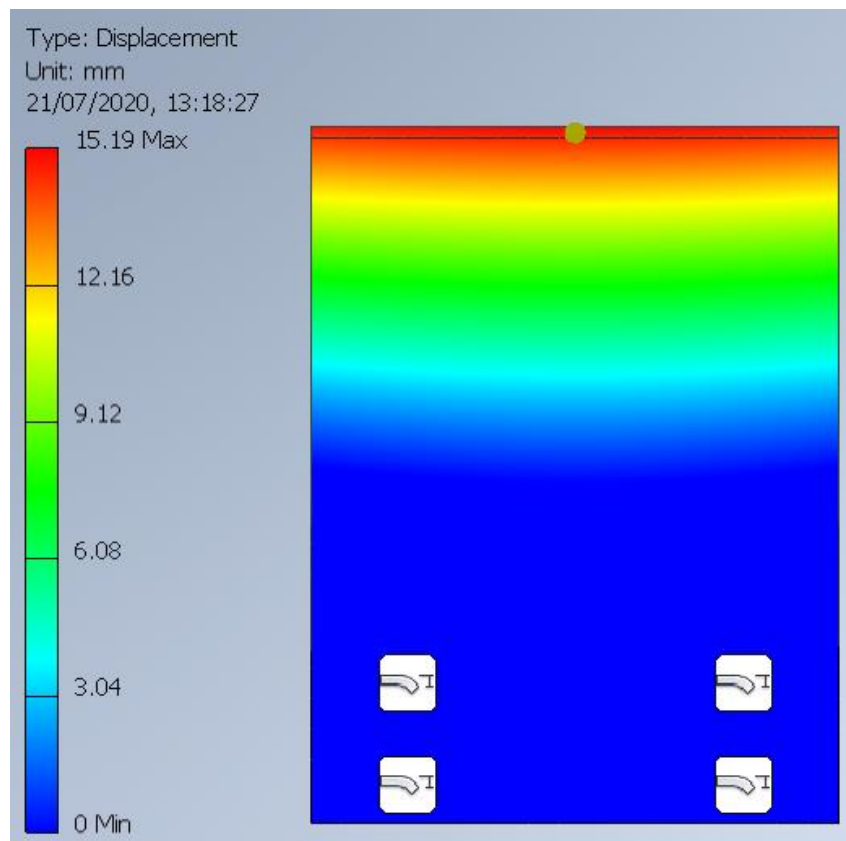
### Glass Analysis - Deflection of Glass Panel due to 1.5kN/m Balustrade Loading:

- Analysis Software was used to determine maximum bending stress of the glass due to 1.5kN/m Balustrade Loading
- Actual Balustrade Load applied to the glass is 1.65 kN/m (1.5x1.1m)
- 10/10/1.52mm T/L/T Glass analysed, horizontally toughened Laminated
- Interlayer Properties used for analysis, E= 606 MPa, G = 203.36MPa Sentry Glass SG5000
- Deflection analysed based on glass panel of 1.1m x 1.45m

### Result:

Max. Deflection = 15.19mm < 25mm {BS6180:2011 cl. 6.4.1}

**OK in Deflection (Glass Only)**



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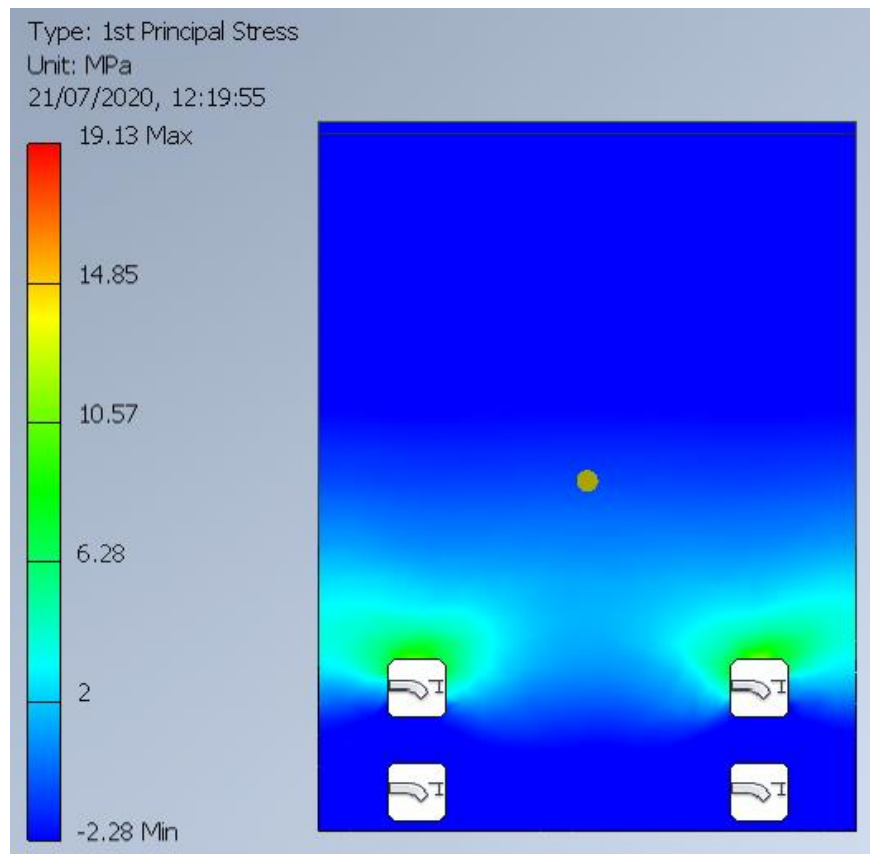
### Glass Analysis - Bending Stress of Glass Panel due to 1.5kN/m Point Load:

- Analysis Software was used to determine maximum bending stress of the glass due to 1.5kN/m Point Load
- 10/10/1.52mm T/L/T Glass analysed, horizontally toughened Laminated
- Interlayer Properties used for analysis, E= 606 MPa, G = 203.36MPa Sentry Glass SG5000
- Bending Stress analysed based on glass panel of 1.1m x 1.45m

### Result:

Max. Bending Stress =  $19.13\text{N/mm}^2 \times 1.5 = 28.70\text{N/mm}^2 < 84.2\text{N/mm}^2$

**OK in Bending**



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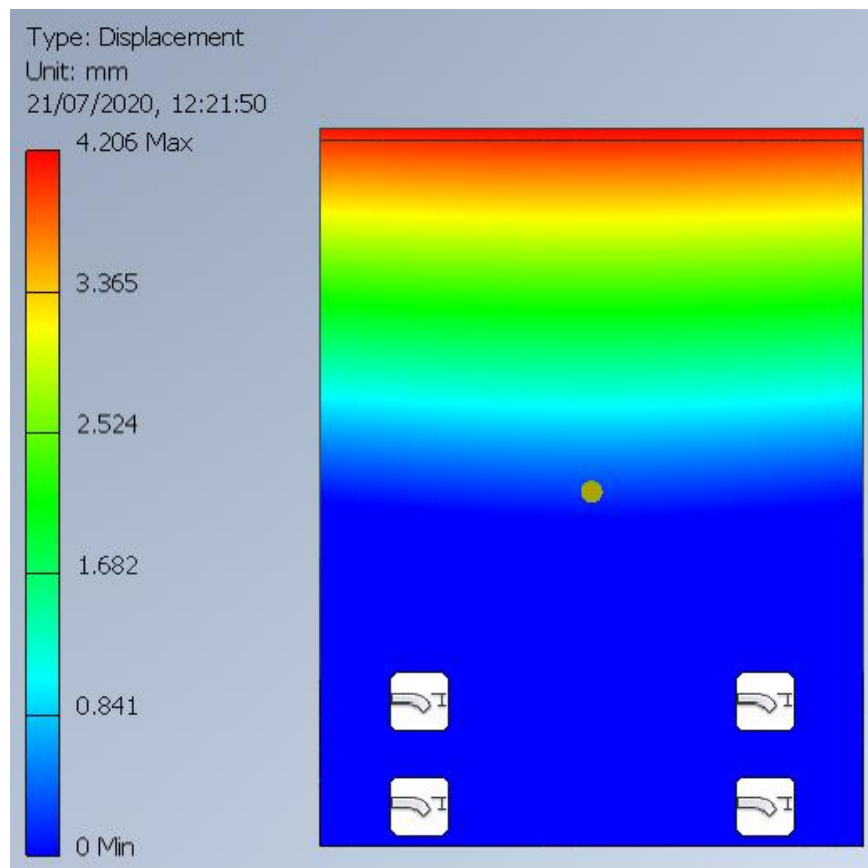
### Glass Analysis - Deflection of Glass Panel due to 1.5kN/m Point Load:

- Analysis Software was used to determine maximum deflection of the glass due to 1.5kN/m Point Load
- 10/10/1.52mm T/L/T Glass analysed, horizontally toughened Laminated
- Interlayer Properties used for analysis, E= 606 MPa, G = 203.36MPa Sentry Glass SG5000
- Deflection analysed based on glass panel of 1.1m x 1.45m

### Result:

Max. Deflection = 4.206mm < 25mm {BS6180:2011 cl. 6.4.1}

**OK in Deflection (Glass Only)**





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Reactions:

Case Study 01			
Size of the Glass – 1100 (l) x 1450 (h) mm			
Reactions (N)			
	Balustrade	Pressure	Point
1	2161	3256	1496
2	2161	3256	1496
3	-1336	-2431	-746
4	-1336	-2431	-746

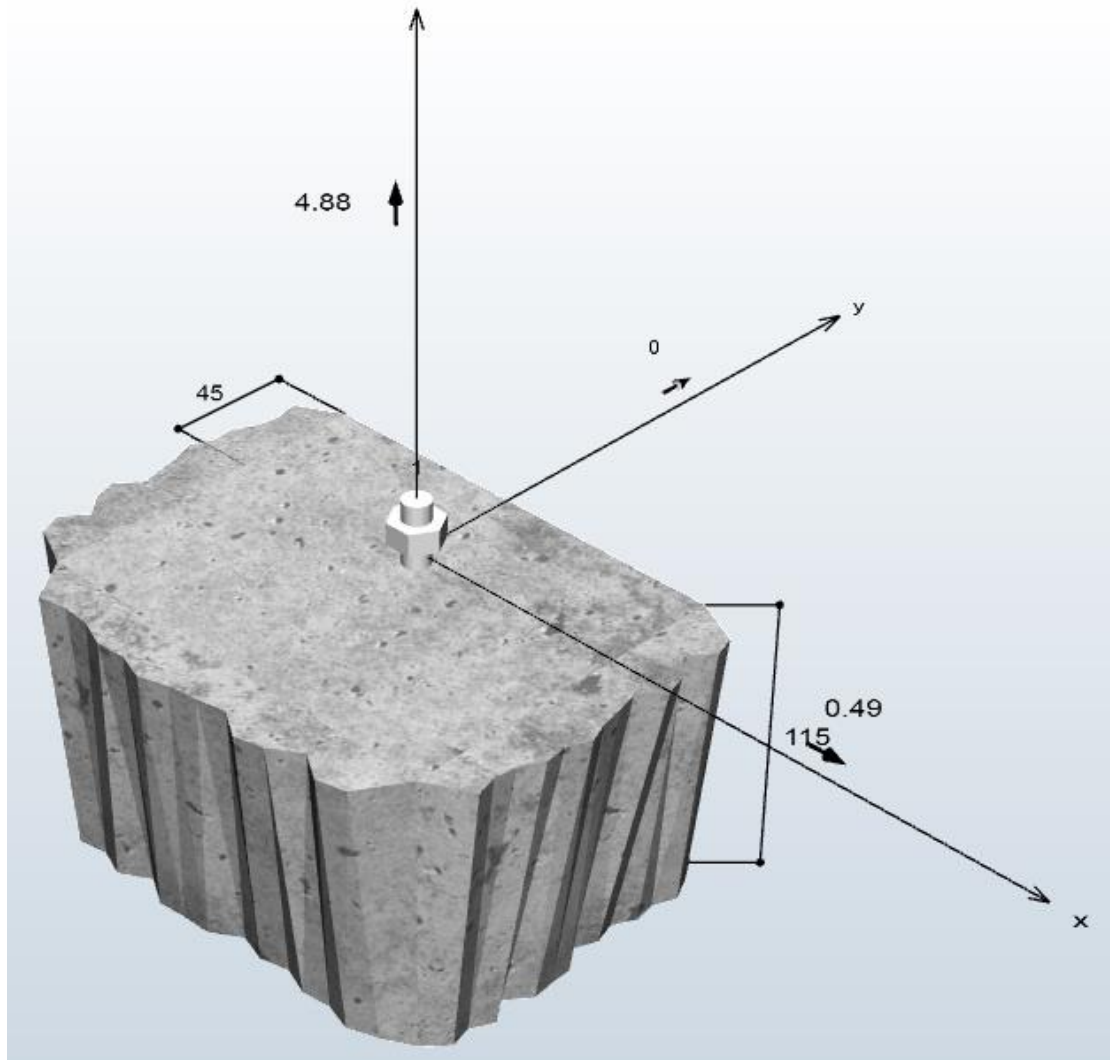
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Connection Design:  
Connection To Concrete:

Tensile Load =  $3.256\text{kN} \times 1.5 = 4.88\text{kN}$  (ULS)

Shear Load =  $0.36\text{kN} \times 1.35 = 0.49\text{kN}$  (ULS)

Therefore use FIS V 360 S Chemical Resin. See design in Appendix A.



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### Connection To Stainless Steel:

1Nr M10 Bolt Grade 316 Stainless Steel

$$f_y = 210 \text{ MPa} \quad (\text{Grade 316 Stainless Steel, Table 2.1 EN 1993-1-4:2006})$$

$$f_{ub} = 520 \text{ MPa} \quad (\text{Grade 316 Stainless Steel, Table 2.2 EN 1993-1-4:2006})$$

$$\alpha = 0.6 \quad (6.2 \text{ EN 1993-1-4:2006})$$

$$A = 58.00 \text{ mm}^2 \quad (\text{For M10 Bolts})$$

$$K_2 = 0.9 \quad (\text{Table 3.4 EN 1993-1-8:2005})$$

$$\lambda_{m2} = 1.25 \quad (\text{Table 5.1 EN 1993-1-4:2006})$$

Tensile Resistance Check: (Table 3.4 EN 1993-1-8:2005)

$F_{t,Ed}$ : is the design tensile force per bolt for the ultimate limit state.

$F_{t,Rd}$ : is the design tension resistance per bolt.

$$F_{t,Ed} = \text{kN}$$

$$F_{t,Rd} = \frac{K_2 F_{ub} A}{\lambda_{m2}} = \frac{0.9 \times 520 \times 58}{1.25} \times 10^{-3} = 14.48 \text{ kN} \rightarrow F_{t,Rd} = 14.48 \text{ kN} > 4.88 \text{ kN} \quad \text{Okay}$$

Shear Resistance Check: (6.2 EN 1993-1-4: 2006)

$F_{v,Ed}$ : is the design shear force per bolt for the ultimate limit state.

$F_{v,Rd}$ : is the design shear resistance per bolt.

$$F_{v,Ed} = \text{kN}$$

$$F_{v,Rd} = \frac{\alpha F_{ub} A}{\lambda_{m2}} = \frac{0.9 \times 520 \times 57}{1.25} \times 10^{-3} = 31.56 \text{ kN} \rightarrow F_{v,Rd} = 31.56 \text{ kN} > 0.49 \text{ kN} \quad \text{Okay}$$

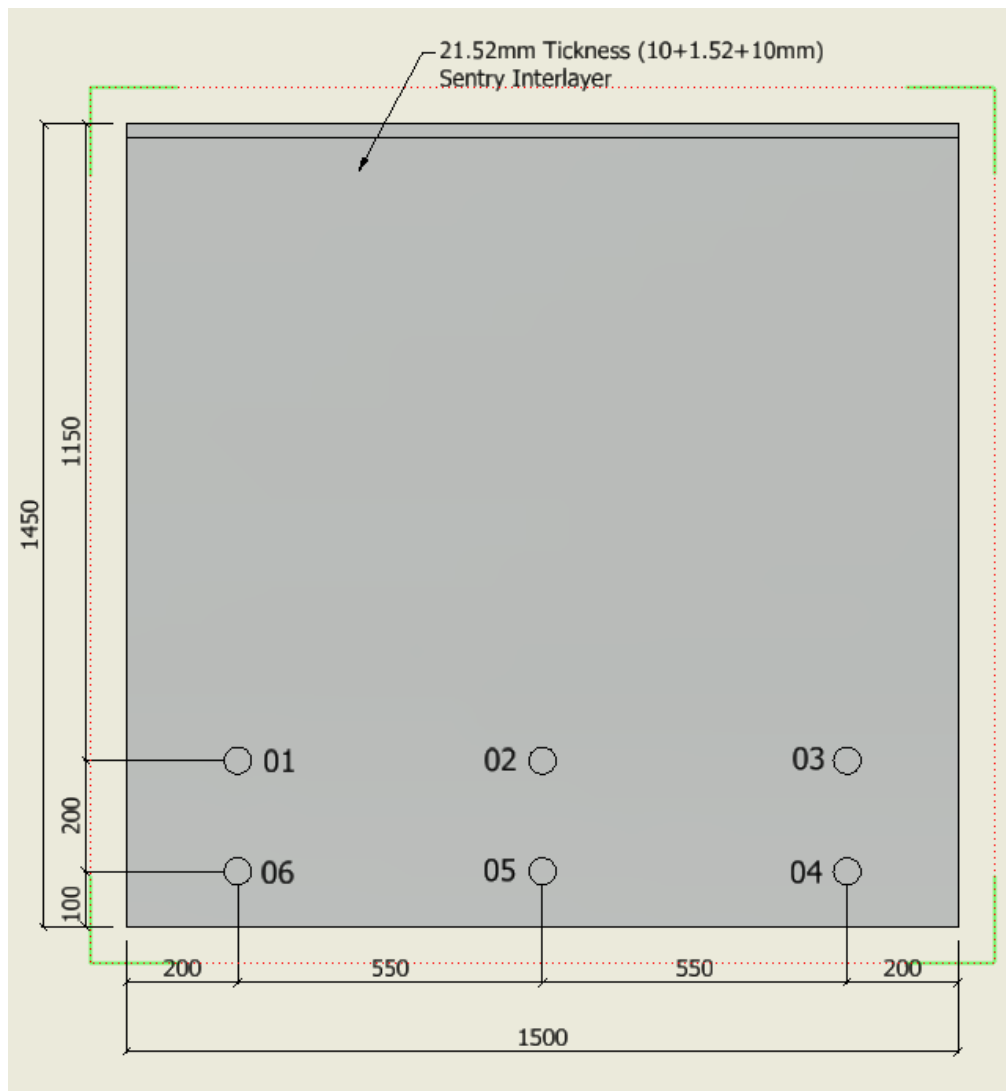
Combined Shear & Tensile Resistance Check: (Table 3.4 EN 1993-1-8:2005)

$$\frac{F_{v,Ed}}{F_{v,Rd}} + \frac{F_{t,Ed}}{1.4 F_{t,Rd}} \leq 1 \rightarrow \frac{4.88}{14.48} + \frac{0.49}{31.56} = \leq 1 \quad \text{Okay}$$

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### Case Study 02:

Sketch - 21.52mm thickness – 1.5kN/m - 1.5x1.45m – Sentry Interlayer:



### NOTE:

- Deflection on the glass 14.42mm = **OK in deflection**

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Glass Analysis – 21.52mm thickness – 1.5kN/m – 1.5x1.45m – Sentry Interlayer:

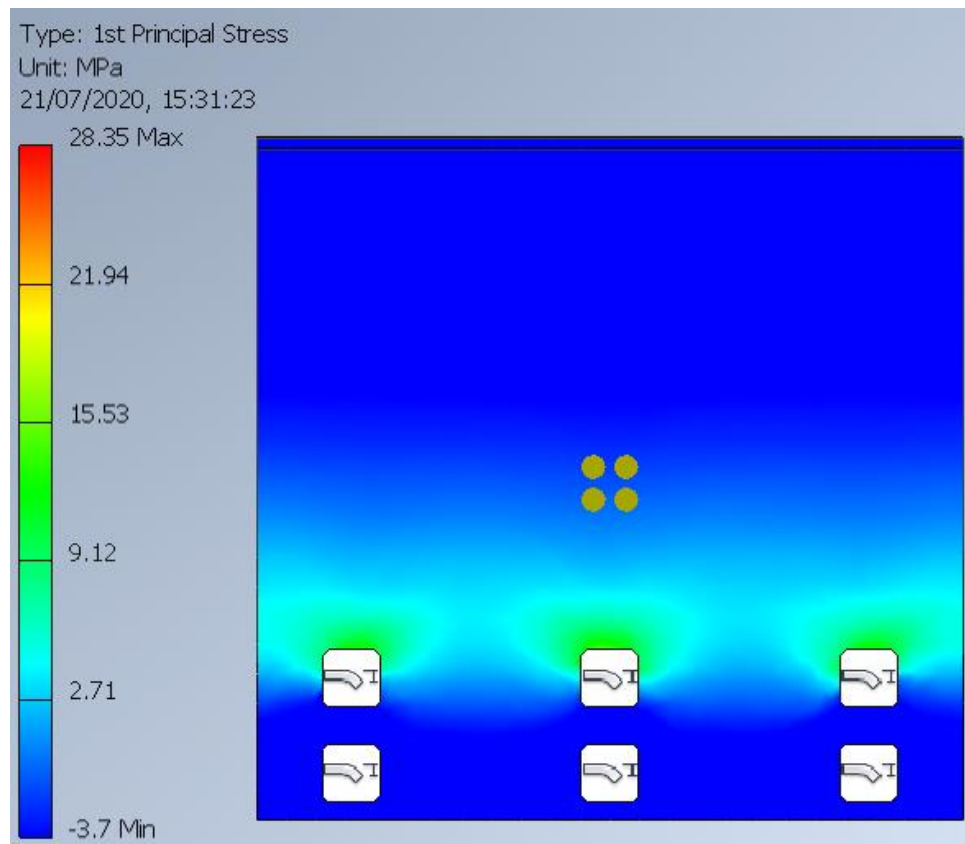
### Glass Analysis - Bending Stress of Glass Panel due to 1.5kN/m<sup>2</sup> Infill Loading:

- Analysis Software was used to determine maximum bending stress of the glass due to 1.5N/m<sup>2</sup> Infill Loading
- 10/10/1.52mm T/L/T Glass analysed, horizontally toughened Laminated
- Interlayer Properties used for analysis, E= 606 MPa, G = 203.36MPa Sentry Glass SG5000
- Bending Stress analysed based on glass panel of 1.5m x 1.45m

### Result:

Max. Bending Stress =  $28.35\text{N/mm}^2 \times 1.5 = 42.53\text{N/mm}^2 < 84.2\text{N/mm}^2$

**OK in Bending**





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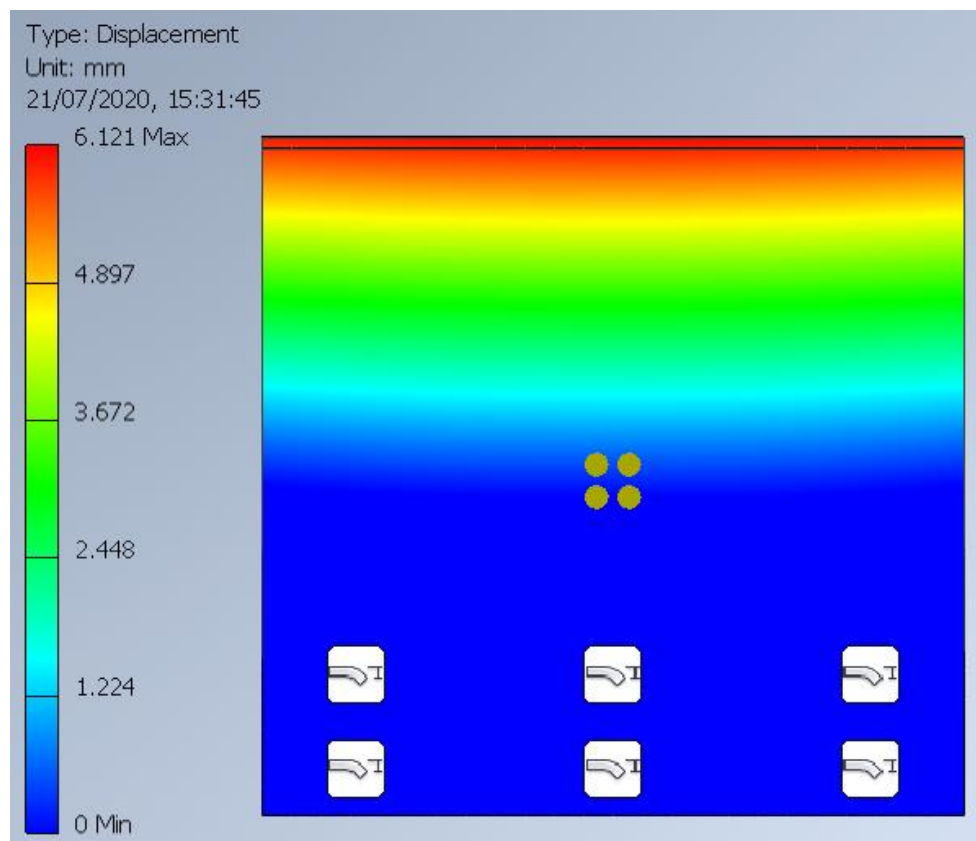
### Glass Analysis - Deflection of Glass Panel due to 1.5kN/m<sup>2</sup> Infill Loading:

- Analysis Software was used to determine maximum bending stress of the glass due to 1.5N/m<sup>2</sup> Infill Loading
- 10/10/1.52mm T/L/T Glass analysed, horizontally toughened Laminated
- Interlayer Properties used for analysis, E= 606 MPa, G = 203.36MPa Sentry Glass SG5000
- Deflection analysed based on glass panel of 1.5m x 1.45m

### Result:

Max. Deflection = 6.121mm < 25mm {BS6180:2011 cl. 6.4.1}

**OK in Deflection (Glass Only)**



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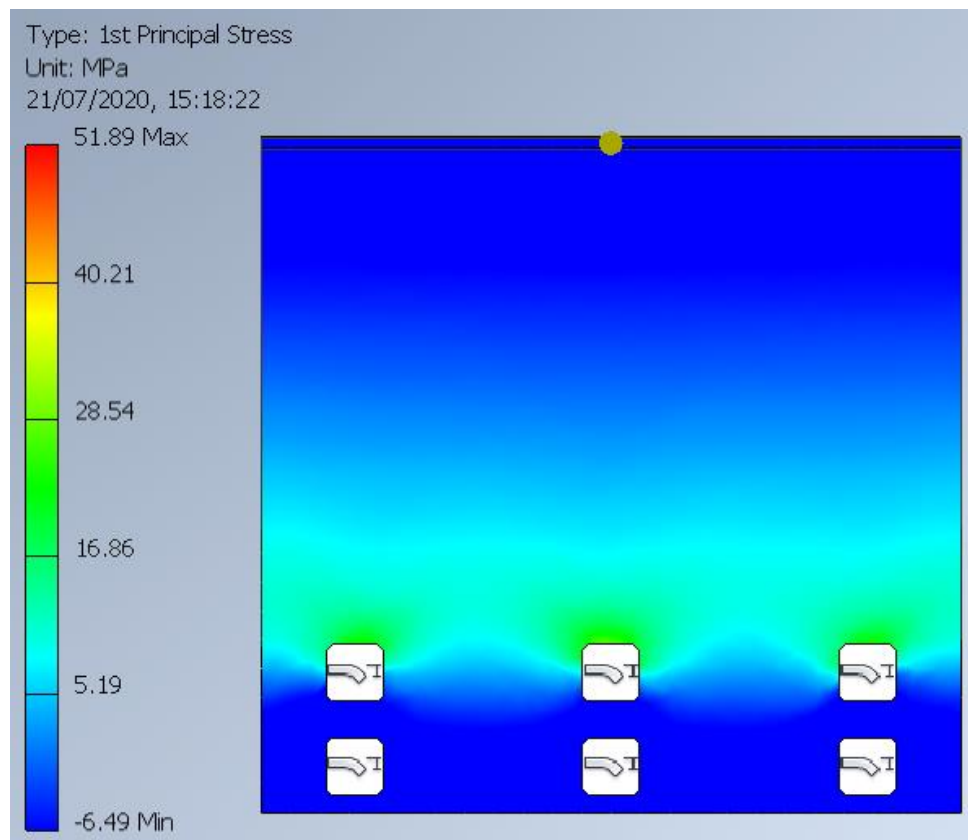
### Glass Analysis - Bending Stress of Glass Panel due to 1.5kN/m Balustrade Loading:

- Analysis Software was used to determine maximum bending stress of the glass due to 1.5kN/m Balustrade Loading
- Actual Balustrade Load applied to the glass is 2.25 kN/m (1.5x1.5m)
- 10/10/1.52mm T/L/T Glass analysed, horizontally toughened Laminated
- Interlayer Properties used for analysis, E= 606 MPa, G = 203.36MPa Sentry Glass SG5000
- Bending Stress analysed based on glass panel of 1.5m x 1.45m

### Result:

Max. Bending Stress =  $51.89\text{N/mm}^2 \times 1.5 = 77.84\text{N/mm}^2 < 84.2\text{N/mm}^2$

**OK in Bending**



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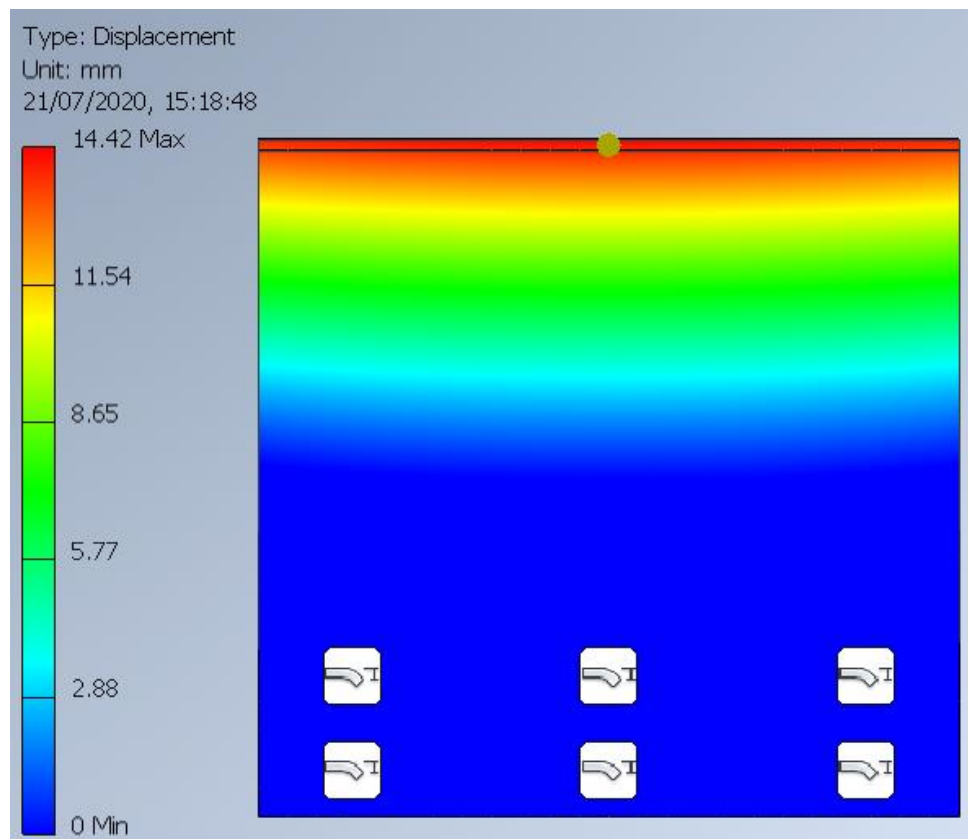
### Glass Analysis - Deflection of Glass Panel due to 1.5kN/m Balustrade Loading:

- Analysis Software was used to determine maximum bending stress of the glass due to 1.5kN/m Balustrade Loading
- Actual Balustrade Load applied to the glass is 2.25 kN/m (1.5x1.5m)
- 10/10/1.52mm T/L/T Glass analysed, horizontally toughened Laminated
- Interlayer Properties used for analysis, E= 606 MPa, G = 203.36MPa Sentry Glass SG5000
- Deflection analysed based on glass panel of 1.5m x 1.45m

### Result:

Max. Deflection = 14.42mm < 25mm {BS6180:2011 cl. 6.4.1}

**OK in Deflection (Glass Only)**



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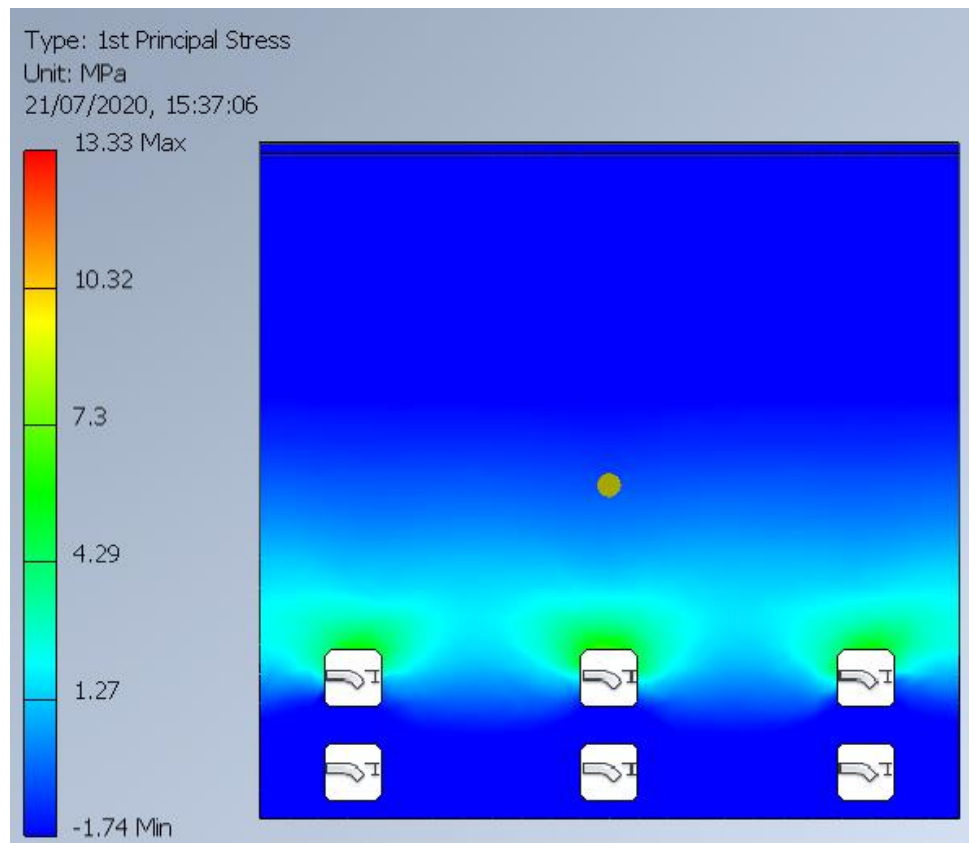
### Glass Analysis - Bending Stress of Glass Panel due to 1.5kN/m Point Load:

- Analysis Software was used to determine maximum bending stress of the glass due to 1.5kN/m Point Load
- 10/10/1.52mm T/L/T Glass analysed, horizontally toughened Laminated
- Interlayer Properties used for analysis, E= 606 MPa, G = 203.36MPa Sentry Glass SG5000
- Bending Stress analysed based on glass panel of 1.5m x 1.45m

### Result:

Max. Bending Stress =  $13.33\text{N/mm}^2 \times 1.5 = 20.00\text{N/mm}^2 < 84.2\text{N/mm}^2$

**OK in Bending**



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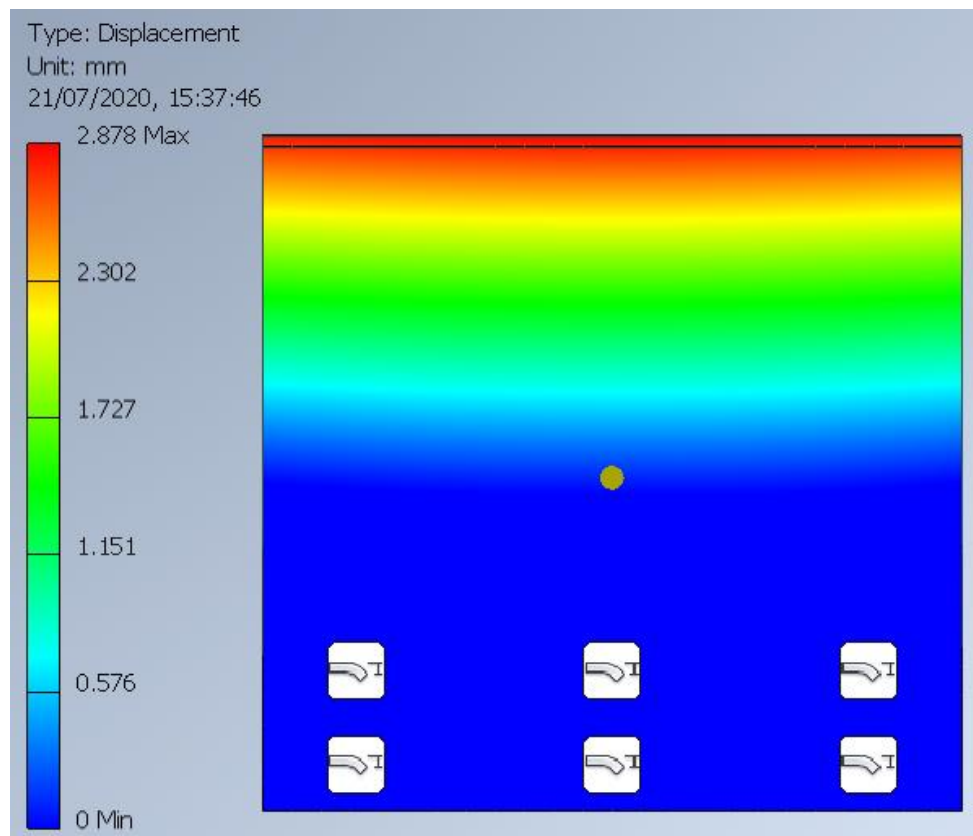
### Glass Analysis - Deflection of Glass Panel due to 1.5kN/m Point Load:

- Analysis Software was used to determine maximum deflection of the glass due to 1.5kN/m Point Load
- 10/10/1.52mm T/L/T Glass analysed, horizontally toughened Laminated
- Interlayer Properties used for analysis, E= 606 MPa, G = 203.36MPa Sentry Glass SG5000
- Deflection analysed based on glass panel of 1.5m x 1.45m

### Result:

Max. Deflection = 2.878mm < 25mm {BS6180:2011 cl. 6.4.1}

**OK in Deflection (Glass Only)**





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Reactions:

Case Study 02			
Size of the Glass – 1500 (l) x 1450 (h) mm			
Reactions (N)			
	Pressure	Balustrade	Point
1	1951	2647	911
2	2505	3726	1171
3	1951	2647	911
4	-981	-2103	-467
5	-1182	-2566	-560
6	-981	-2103	-467

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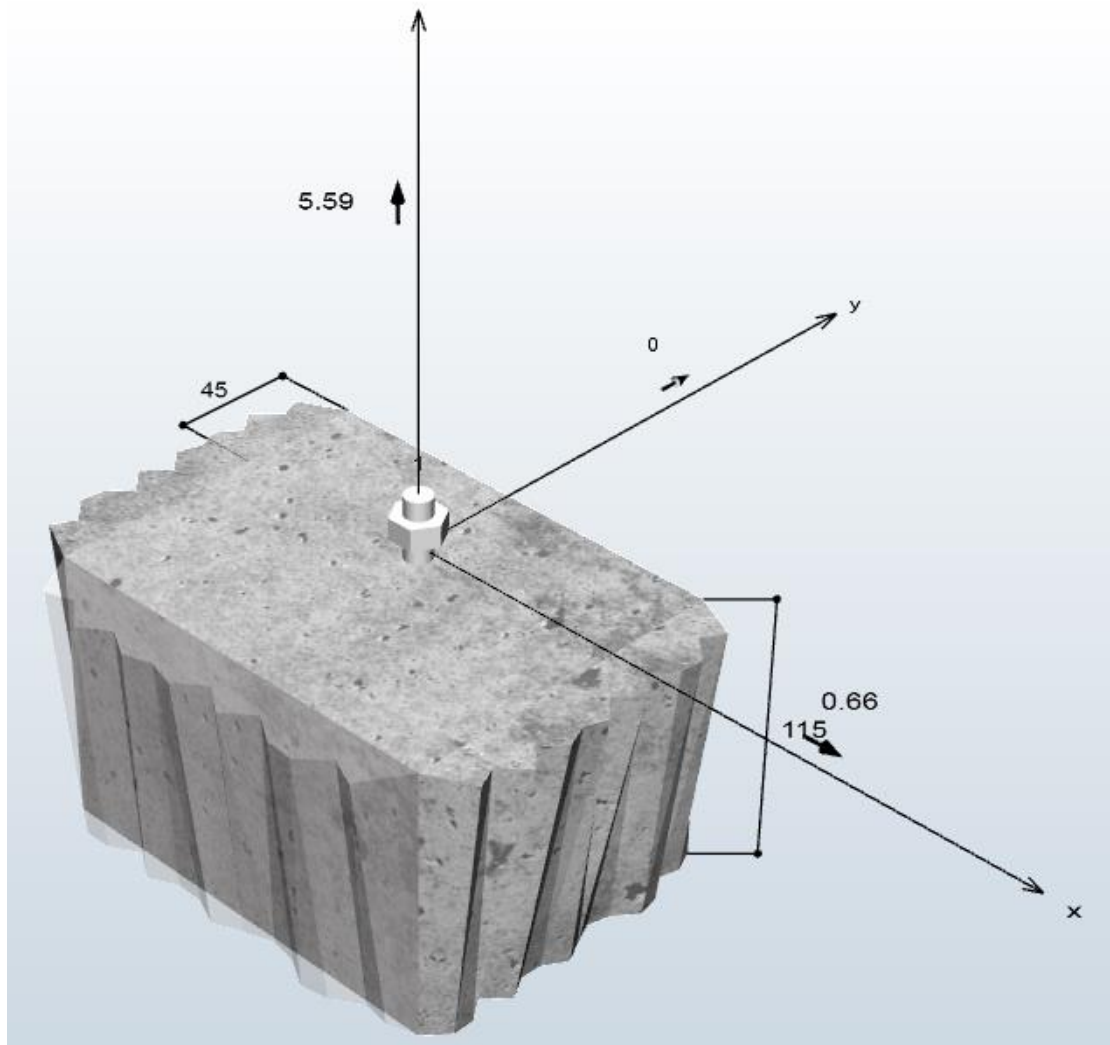
Connection Design:

Connection To Concrete:

Tensile Load =  $3.726\text{kN} \times 1.5 = 5.59\text{kN}$  (ULS)

Shear Load =  $0.49\text{kN} \times 1.35 = 0.66\text{kN}$  (ULS)

Therefore use FIS V 360 S Chemical Resin. See design in Appendix A.



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### Connection To Stainless Steel:

1Nr M10 Bolt Grade 316 Stainless Steel

$$f_y = 210 \text{ MPa} \quad (\text{Grade 316 Stainless Steel, Table 2.1 EN 1993-1-4:2006})$$

$$f_{ub} = 520 \text{ MPa} \quad (\text{Grade 316 Stainless Steel, Table 2.2 EN 1993-1-4:2006})$$

$$\alpha = 0.6 \quad (6.2 \text{ EN 1993-1-4:2006})$$

$$A = 58.00 \text{ mm}^2 \quad (\text{For M10 Bolts})$$

$$K_2 = 0.9 \quad (\text{Table 3.4 EN 1993-1-8:2005})$$

$$\lambda_{m2} = 1.25 \quad (\text{Table 5.1 EN 1993-1-4:2006})$$

Tensile Resistance Check: (Table 3.4 EN 1993-1-8:2005)

$F_{t,Ed}$ : is the design tensile force per bolt for the ultimate limit state.

$F_{t,Rd}$ : is the design tension resistance per bolt.

$$F_{t,Ed} = \text{kN}$$

$$F_{t,Rd} = \frac{K_2 F_{ub} A}{\lambda_{m2}} = \frac{0.9 \times 520 \times 58}{1.25} \times 10^{-3} = 14.48 \text{ kN} \rightarrow F_{t,Rd} = 14.48 \text{ kN} > 5.59 \text{ kN} \quad \text{Okay}$$

Shear Resistance Check: (6.2 EN 1993-1-4: 2006)

$F_{v,Ed}$ : is the design shear force per bolt for the ultimate limit state.

$F_{v,Rd}$ : is the design shear resistance per bolt.

$$F_{v,Ed} = \text{kN}$$

$$F_{v,Rd} = \frac{\alpha F_{ub} A}{\lambda_{m2}} = \frac{0.9 \times 520 \times 57}{1.25} \times 10^{-3} = 31.56 \text{ kN} \rightarrow F_{v,Rd} = 31.56 \text{ kN} > 0.66 \text{ kN} \quad \text{Okay}$$

Combined Shear & Tensile Resistance Check: (Table 3.4 EN 1993-1-8:2005)

$$\frac{F_{v,Ed}}{F_{v,Rd}} + \frac{F_{t,Ed}}{1.4 F_{t,Rd}} \leq 1 \rightarrow \frac{5.59}{14.48} + \frac{0.66}{31.56} = \leq 1 \quad \text{Okay}$$





<b>Project:</b> Glass Adaptor 1.5kN	<b>Contract:</b> 1419-1
<b>Subject:</b> Glass Balustrade	<b>Sheet No.</b> 25
<b>Date:</b> 23/07/2020	<b>By:</b> R.F.

## Appendix A - Fiscer Reports

TSA is Both the Designer and the Specifier of the Fixings.



<b>Project:</b> Glass Adaptor 1.5kN	<b>Contract:</b> 1419-1
<b>Subject:</b> Glass Balustrade	<b>Sheet No.</b> 26
<b>Date:</b> 23/07/2020	<b>By:</b> R.F.

Appendix B – Glass Adaptor ø 50mm